

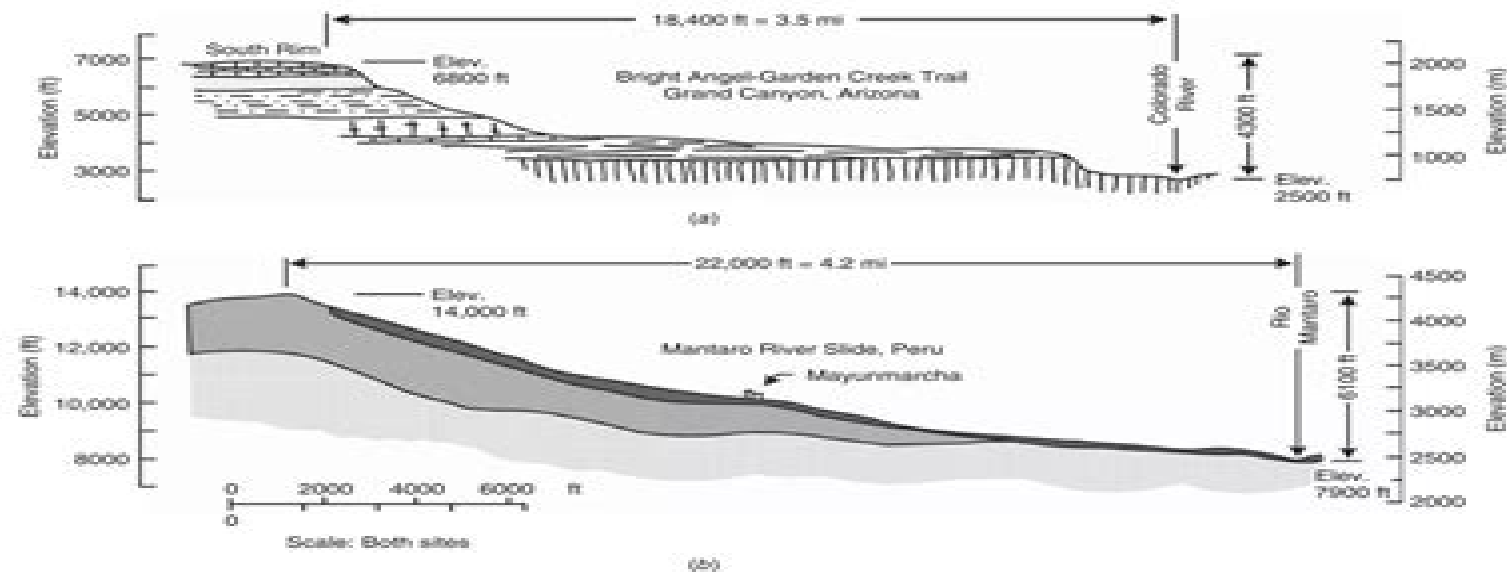


Figure 2.15 Comparison of cross sections: (a) Grand Canyon, Arizona and (b) Mantaro River landslide.

It was estimated that the sliding mass achieved a velocity of 120 miles per hour as it moved down the slope. When it slammed into the opposite side of the valley, it splashed up to a height of 600 ft above the bottom of the valley and then slumped back to form a landslide dam about 550 ft high. The impact as the sliding mass hit the opposite valley wall was recorded at a seismographic station 30 miles away as an event comparable to a Magnitude 4 earthquake.

Prior to the slide a town of about 450 inhabitants, Mayunmarca, was situated on the slope where the landslide occurred. After the landslide no trace was found of the town or its inhabitants.

2.6 KETTLEMAN HILLS LANDFILL FAILURE

On March 19, 1988, a slide occurred in a 90-ft-high slope of a hazardous-waste landfill at Kettleman Hills, California (Mitchell et al., 1990; Seed et al., 1990). The failure involved about 580,000 yd³ of solid waste (Golder Associates, 1991). A plan view and cross section through the fill are shown in Figure 2.16. The failure occurred by sliding on interfaces within the composite liner beneath the waste. The liner included three geomembranes, six geotextiles, three layers

of granular fill, and two layers of compacted clay. Mitchell et al. (1990) found that some of the interfaces within the liner system had interface friction angles as low as 8 degrees. The lowest friction angles were found for interfaces between high-density polyethylene (HDPE) geomembranes and geotextiles, between geomembranes and geonets, and between geomembranes and compacted clay that had become saturated after compaction. Seed et al. (1990) showed that factors of safety calculated for the conditions at failure were near 1.0 if the effect of wetting of the lower, nearly flat, portion of the liner was taken into account and if consideration was given to three-dimensional effects. One particularly interesting aspect of the failure is that the maximum section (C1-C2 in Figure 2.16) did not have the lowest factor of safety. A shallower section near the top of Figure 2.16 had a considerably smaller factor of safety (Seed et al., 1990).

2.7 CAUSES OF SLOPE FAILURE

It is important to understand the agents of instability in slopes for two reasons. First, for purposes of designing and constructing new slopes, it is important to be able to anticipate the changes in the properties of the soil within

Soil Strength And Slope Stability

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Table of Contents Soil Strength And Slope Stability

1. Understanding the eBook Soil Strength And Slope Stability
 - The Rise of Digital Reading Soil Strength And Slope Stability
 - Advantages of eBooks Over Traditional Books
2. Identifying Soil Strength And Slope Stability
 - Exploring Different Genres
 - Considering Fiction vs. Non-Fiction
 - Determining Your Reading Goals
3. Choosing the Right eBook Platform
 - Popular eBook Platforms
 - Features to Look for in an Soil Strength And Slope Stability
 - User-Friendly Interface
4. Exploring eBook Recommendations from Soil Strength And Slope Stability
 - Personalized Recommendations
 - Soil Strength And Slope Stability User Reviews and Ratings
 - Soil Strength And Slope Stability and Bestseller Lists
5. Accessing Soil Strength And Slope Stability Free and Paid eBooks
 - Soil Strength And Slope Stability Public Domain eBooks
 - Soil Strength And Slope Stability eBook Subscription Services
 - Soil Strength And Slope Stability Budget-Friendly Options
6. Navigating Soil Strength And Slope Stability eBook Formats

- ePub, PDF, MOBI, and More
- Soil Strength And Slope Stability Compatibility with Devices
- Soil Strength And Slope Stability Enhanced eBook Features
- 7. Enhancing Your Reading Experience
 - Adjustable Fonts and Text Sizes of Soil Strength And Slope Stability
 - Highlighting and Note-Taking Soil Strength And Slope Stability
 - Interactive Elements Soil Strength And Slope Stability
- 8. Staying Engaged with Soil Strength And Slope Stability
 - Joining Online Reading Communities
 - Participating in Virtual Book Clubs
 - Following Authors and Publishers Soil Strength And Slope Stability
- 9. Balancing eBooks and Physical Books Soil Strength And Slope Stability
 - Benefits of a Digital Library
 - Creating a Diverse Reading Collection Soil Strength And Slope Stability
- 10. Overcoming Reading Challenges
 - Dealing with Digital Eye Strain
 - Minimizing Distractions
 - Managing Screen Time
- 11. Cultivating a Reading Routine Soil Strength And Slope Stability
 - Setting Reading Goals Soil Strength And Slope Stability
 - Carving Out Dedicated Reading Time
- 12. Sourcing Reliable Information of Soil Strength And Slope Stability
 - Fact-Checking eBook Content of Soil Strength And Slope Stability
 - Distinguishing Credible Sources
- 13. Promoting Lifelong Learning
 - Utilizing eBooks for Skill Development
 - Exploring Educational eBooks
- 14. Embracing eBook Trends
 - Integration of Multimedia Elements
 - Interactive and Gamified eBooks

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