#### Security and a

 Atlans, stiffness, and rutural period of the SEXOF systems. Since we assumed the beam to the rigid, the stiffness of the system is given by the sum of column lateral stiffnesses. The mass and stiffness of the SEXOF system are calculated as follows: Atlans.

$$m = \frac{W}{g} = \frac{1500 \text{ Brs}}{386.4 \text{ in/s}^2} = 3.88 \text{ Br-s}^2/m$$

Suffices type Table 3.1), when both columns are fixed-pioned.

$$k = \frac{MT}{L^2} = \frac{MT}{L^2} = 2 \left[ \frac{3(29,0000,0000 \, \mathrm{pulk} 75 \, \mathrm{im}^4)}{(5.5 \, \mathrm{ft} - 12 \, \mathrm{im} 40)^2} \right] = 22.37.6 \, \mathrm{fts/fer}$$

The natural period of the system is

$$T_{\nu} = 2\pi \sqrt{\frac{m}{k}} = 2\pi \sqrt{\frac{3.88 \, \text{fb} \, \text{s}^{3.68}}{22.37.6 \, \text{fb} \, \text{fb}}} = 0.262 \, \text{s}$$

 Determine the maximum displacement for ductility of 4; u<sub>c</sub>: First determine A due to an 84.7% design spectrum, scaled to 0.5g PGA, by entering the response spectrum in Figure 3.22 with F<sub>c</sub> = 0.26 s, which is in the acceleration sensitive region.

$$A = q \cdot 2.7 \text{ kg} = 0.5(2.7 \text{ kg}) = 1.335 \text{ g}$$

The maximum displacement is computed using figureion 5.28 and Figure 5.18.

$$\omega_{n} = \mu \left(\frac{E_{n}}{2\pi}\right)^{2} A_{n} = \mu \left(\frac{E_{n}}{2\pi}\right)^{2} \frac{A}{\sqrt{2\mu + 1}} = 4 \left(\frac{0.262 \, s}{2\pi}\right)^{2} \frac{1.335(386.4 \, in/6^{2})}{\sqrt{2\mu 4} - 1} = 1.353 \, in$$

 Determine the required column strength for a ductility of 4, 7,: To determine F<sub>p</sub> use A = 1.305g and figuration 5.29.

$$I_{\nu} = mA_{\nu} = \frac{mA}{\sqrt{2\mu - 1}} = \frac{3.888 \text{ fb/s}^2 / \text{fe/(1.355)/(365.4 in/s)}/4}{\sqrt{2(4\mu - 3)}} = 270 \text{ fb/s}$$

#### PROCEEDINGS.

- 5.1 Use the MATLAB script provided in Chapter 4, Example 4.13 to draw suspense spectra for the displacement, velocity, and acceleration for an SDOF system subjected to the 1994 Northridge earthquake ground acceleration for 2%, 5%, and 10% damping.
- 5.2 The building frame shown is subjected to the EI Centro earthquake; determine the total stiffness, structural period, deflection of the beam, base shear, and bending stresses in each of the columns. The roof weighs 10 kips and the substructure has 5% damping, modulus of elasticity, E = 29,000 ksi, and the other geometric properties shown.

# **Solutions Manual For The Dynamics Of Heat**

**Hussin A.Rothana** 

#### **Solutions Manual For The Dynamics Of Heat:**

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